

# Supplementary Information Tolerances Structural & Battery

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# General Disclaimer

Since 1988, A1 Shutters Limited have manufactured, installed and exported fire curtains, fire shutters and industrial shutters. Whilst we hold over 50 testing documents and hold some of the highest global certification available, our products should only be specified by qualified architects, fire strategists design managers or local Building Control Organizations who are considered competent professionals within what is a complex and highly regulatory area. As such, whilst we can outline our product performances A1 Shutters Limited cannot guarantee that our products are appropriate for any given application.

# **Battery Back Up**

In the event of onsite power failure, the Fire Curtains are designed to stay in the open position for a period of between 2 & 4 hours due to the trickle feed from the fire control panel to the batteries. Should the mains power not be reinstated in time, the batteries will deplete of power resulting in the fire curtains dropping under controlled descent. If this is the case then the batteries will need to be replaced and the fire control panel will need resetting. This would be a chargeable visit as the mains power supply is not covered under the terms of the warranty.



















# Fire Shutters & Curtains Fire Products Structural Recommendation

The structure supporting the rolling door-set must be a fire resistant separating element to BS EN 1634-1 2014, or BS 476-22 having a fire resistance greater or equal to that required of the door-set itself. It must be capable of supporting the door-set for the required fire resistance period without compromising the fire performance of the door-set. In addition, consideration must be given to the loads imposed by the various door-set components, i.e. end plates and barrel. Our test documentation requires that a critical temperature of 400 degrees centigrade and steel sections with an Hp/A value below 230 m -1 are used for designing fire resistant steel supporting sections. (see overleaf) Structure and supporting structure is by others and as such should be designed by competent professionals.

Where fire rated rolling shutters are fixed to masonry supporting elements, the masonry elements must be capable of providing a fire resistance in terms of the insulation, integrity and load bearing capacity criteria of 1634-1 equal to or greater than the fire resistance rating (integrity) of the rolling shutter assembly.

Fire shutters fitted to a timber stud partition / steel framed partition are acceptable for up to 60 minutes fire resistance, to the tested opening size of 2500 mm wide X 2400 mm high and above such sizes are subject to construction within dictates of our Extended Application to BS EN 15269-10. Indicative parameters are available at https://fireshutter.co.uk/

The surface/cill onto which the shutter closes must be composed of noncombustible material providing a fire resistance equal to or greater than that required form the shutter

All openings must be capable of carrying the weight of the shutter and be equal or greater than the fire resistance of the roller shutter assembly. All roller shutter weights are indicated on the approval drawings.

Safety Bearings (Fire Shutters) – Risk Assessment

All A1S Fire Shutters are tested with safety bearing which come as a standard.

Fire Alarm Requirements

**Tube motor fire shutter** - The relay provided by A1 requires a normally open volt free signal going closed on fire.

**Conventional fire shutter -** The solenoid unit requires a 24-volt D.C. signal (0.5 amps on activation)

Audio Visual unit - Requires a normally open volt free signal going closed on fire.

Fire Curtains - Require a normally closed signal going open on fire (fail-safe system)

















# Additional Structural Comments For Fire Rated Shutters

## J.2 Modified Supporting Construction

J.2.1 Change from standard supporting construction to protected structural steel supporting construction

Possible providing the following applies

- A) Structural steel section factor, A/V must be less than 230m-1. Section factor to be calculated assuming section is exposed to fire on all four sides. The section factor shall be calculated as described in EN 13381-4 and EN 13381-8. This rule applies to both the vertical and the horizontal steel sections of the support frame.
- B) Fire protection system must have been shown by test to EN 13381 to maintain the steel temperature 400°C or less to retain strength and minimise the effects of expansion in the steel section.
- C) The fixings securing the door to the structural steel shall be in accordance with the appended tables and must be fabricated from steel and designed not to reduce the fire performance of the steelwork fire protection system in ambient conditions or in fire conditions.

Possible to install into standard rigid supporting construction (as tested) or to install into protected structural steelwork subject to specified conditions.

Note: Also tested within timber stud wall and a separate scope of approval for this application is provided in the tables appended to this report (wall specification to be as tested under WF No. 421972)

# K Decorative and/or protective finishes.

K.1.1 And paint finish

Possible in line with direct application

Paint finish may be added to the shutter components.



















## **EXPECTED STRUCTURAL SITE CONDITION TOLERANCES**

Structural Concrete

Expected Structural Tolerances

of Concrete Frame

#### Structural Concrete

**Expected Structural Tolerances** of Concrete Frame (National Structural Concrete Specification

#### Overall Structure

Levels Level of floors measured relative to the intended design level at reference level

Permitted deviation  $\Delta$ 

H ≤ 10m = 15mm 10m < H < 100m = 0.5(H + 20)mm H 100m = 0.2(H = 200)mm

Inclination Permitted deviation

= the smaller of 50mm or H/(200n½)mm where h = free storey height in mm H = free height at location =  $\Sigma$ h, in mm

#### Structural Concrete

**Expected Structural Tolerances** of Concrete Frame for Building Construction 4th Edition)

Base Support (foundations)

Base supports shall include direct foundations and pile caps, etc but not deep foundations such as piles, slurry walls, diaphragms, special anchorages, etc.

Permitted deviation in any direction

= 25mm Vertical Section

Position in the vertical direction of a base support relative to the intended design position

Permitted deviation Δ = 20mm where supporting concrete superstructure. Permitted deviation  $\Delta = -15$ mm + 5mm

where supporting steel superstructure.

where h = intended distance of foundation below datum level.

#### Structural Concrete

Expected Structural Tolerances of Concrete Frame

## Foundation Bolts & Similar Inserts

Note: Deviations are coordinated with BS EN 1090-2: 2008 CL.D.2.20

Preset Bolt Prepared For Adjustment

Distance of centre of here

Distance of centre of bolt group from intended design postion. Permitted deviation = 6mm

Location of boll at tip, from centre of boll group. Permitted deviation  $\Delta y_1$   $\Delta z = 10$ mm

Permitted deviation Δp: -5mm Δp≤ 25mm

Preset Foundation Bolt Not Prepared For Adjustment

Distance of centre of bolt course.

stance of centre of bolt group from intended design postion

Permitted deviation = 3mm Location of bolt at tip, from centre of bolt group.

Permitted deviation  $\Delta y_1 \Delta z = 3mn$ Vertical protrusion.

- 5mm  $\leq \Delta_p \! \leq$  45mm Permitted deviation  $\Delta p$ :

Horizontal protrusion. Permitted deviation  $\Delta x$ : - 5mm  $\leq \Delta p \leq$  45mm

Structural Concrete

Expected Structural Tolerances of Concrete Frame (National Structural Concrete Specification for Building Construction 4th Edition)

## Elements - Beams & Slabs

Horizontal straightness of beams. Permitted deviation Δ = the larger of 15mm or I/600mm where I = distance between supports

Distance Between Adjacent Beams

Distance between adjacent beams, measured at corresponding

Permitted deviation  $\Delta$  = the larger of 20mm, but not more than

where I = distance between support centre lines in mm

Inclination Of Beam or Slab

Difference in level across a beam or slab at corresponding points

Permitted deviation  $\Delta = (10 + t/500)$ mm where I =span of element in mm

sition of Slab Edge

Position of slab edge relative to actual slab edge position on the floor below.

Permitted deviation  $\Delta = 10 \text{mm}$ 

### Structural Concrete

Expected Structural Tolerances of Concrete Frame (National Structural Concrete Specification for Bu Iding Construction 4th Edition)

Elements - Columns & Walls

The deviation or sum of any deviations of any individual element must not exceed the overall building structure tolerance.

Position On Plan

Position of the element centre line relative to:

At base level the intended design position.

At any upper level the actual location of the element at the level

Permitted deviation  $\Delta = 10$ mm where I = distance to centre line from gridline.

Verticality By Storey Of The Structure

nn or wall at any level in a single or multi-slorey building.

 $h \le 10m$  = the larger of 15mm or h/100h > 10m = larger of 25mm or h/600 where h = height of element in mm

Offset Between Floors

Deviation between centrelines at floor level mitted deviation  $\Delta$  =targer of 10mm or t/30mm, but not more

than 20mm where  $t = thickness in mm = (t_1 + t_2)/2$ 

Curvature Between Adjacent Floors Curvature of an element between adjacent storey levels.

Permitted deviation Δ for 10m = the larger of 15mm or h/400 h > 10m = larger of 25mm or h/600 where h = height of element in mm

Level Per Storey of Structure

Level of adjacent floors at supports Permitted deviation Δ = 10mm where h = storey height in mm

Distance Between Adjacent Columns & Walls

Permitted deviation  $\Delta$  = the larger of 20mm or 1/600, but not more than 40mm where I = distance between centre lines in mm

Structural Concrete

**Expected Structural Tolerances** of Concrete Frame (National Structural Concrete Specification for Building Construction 4th Edition)

## Section of Elements

Cross-section Dimensions of Elements

ns and other elements coveri

Permitted deviation A for

I ≤ 150mm = 15mm I = 400mm = 15mm

I 400mm = 30mm with linear interpolation for intermediate values

where I = intended dimensions in mr

Squareness of Element

Applicable to beams, stabs, columns and other elements Permitted deviation ∆ = larger of 10mm or a/25mm, but not more

I = 400mm = 15mm

I ≥ 400mm = 30mm

Surface Straightness

Flatness of surface of any element. Basic unformed surface (CI 8.6.2.1)

Permitted global deviation  $\Delta = 12mm$ Permitted local deviation  $\Delta = 5 \text{mm}$ Ordinary unformed surface (Cl 8.6.2.2)

Permitted global deviation ∆ = 9mm Permitted local deviation  $\Delta = 3 \text{mm}$ 

Ordinary surfaces (CI 8.6.1.2)

Permitted global deviation Δ = 9mm

Permitted local deviation  $\Delta = 5 \text{mm}$ Plain surfaces (Cl 8.6.1.3)

Permitted global deviation  $\Delta = 9 \text{mm}$ 

Edge Straightness Straightness of edge of floor slab or element

Permitted deviation  $\Delta \cong$  larger of 10mm or a/25mm, but not more

I ≤ 150mm = 15mm

I = 400mm = 15mm where a = length in mm

(National Structural Concrete Specification for Building Construction 4th Edition) Surface Straightness

Flatness Flainess of surface of any element

Basic unformed surface (Cl 8.6.2.1)

Permitted global deviation  $\Delta = 12$ mm

Permitted local deviation  $\Delta = 5 \text{min}$ Ordinary unformed surface (CI 8.6.2.2)

Permitted global deviation  $\Delta = 9mm$ Permitted local deviation  $\Delta = 3mm$ 

Ordinary surfaces (CI 8.6.1.2) Permitted global deviation  $\Delta = 9$ mm Permitted local deviation  $\Delta = 5$ mm Plain surfaces (Cl 8.6.1.3)

Permitted global deviation  $\Delta = 9 \text{mm}$ 

Permitted local deviation  $\Delta = 3$ mm

 $\frac{\text{Edge Straightness}}{\text{Straightness of edge of floor slab or element.}}$  Permitted deviation  $\Delta$  = larger of 10mm or a/25mm, but not more

than 20mm I ≤ 150mm = 15mm

I = 400mm = 15mm I ≥ 400mm = 30mm

where a = length in mm

Structural Steelwork

Expected Structural Tolerances of Erected Components (National Structural Steelwork Specification for Building Construction 5th Edition)

Position of Columns at Base

Deviation of section centre line from the specified position

= 10mm in either direction

Level of Columns at Base

Deviation of the top of the base plate from specified level

= +/- 5mm Single storey Columns Plumb

Deviation of top releative to base, excluding portal frame columns on main axes.

= +/- H/600 or 5mm whichever is greater. Max = +/-25m Multi-storey Columns Plumb

Deviation in each storey and maximum deviation relative to base

for up to 10 storeys. = +/- H/600 or 5mm whichever is greater. Max to 10 storeys = 50mm

Alignment of Adjacent Perimeter Columns (critical face)
Deviation relative to next column on a line parallel to the grid line
when measured at base or splice.

= 10mm Beam Level

Deviation from specified level at supporting column. = +/- 10mm Level at Each End of Same Beam

Relative deviation in level at ends

= 5mm Beam Alignment

Horizontal deviation relative to an adjacent beam above and

# **MANUFACTURING TOLERANCES COMPONENTS & FINISHES**

### A1S Manufacturing Tolerances <u>Finishes</u>

All finishes to be applied in accordance with A15 issued drawings All finishes to be applied in accordance with A15 issued drawings and specification and in any event to meet with the following and and specification and in any event to meet with the following and British Standards and good practice current at the time of British Standards and good practice current at the time of execution

Any discrepancies between A15 drawings and referred British: Standards to be notified to A15 before manufacture

Applicable British Standards

Polyester Powder Coating Steet

BS 6497 ISO 13438:2005

Polyester Powder Coating Steel e been met priorto

Any discrepancies or quality issues of the galvanising to be notified to A1S before application of powder coating finish.

Minimum Coating Thickness 40 micron Non marine environment 60 micron Marine environment 60 micron Aggressive environments White coatings 60 micron

Fabrications should be coaled to the correct standard and should be free from dents, scratches, runs and over spray and should have a smooth consistent finish,

In the case of a powder coated finish being applied of hot dipped galvanised steel, the finish of the powder coating will be directly affected and proportional to the surface finish and uniformity of the

This is an inherent condition and cannot be avoided.

smooth consistent finish.

# A1S Manufacturing Tolerances

<u>Finishes</u>

Anydiscrepancies between A15 drawings and referred British Standards to be notified to A15 before manufacture.

**Applicable British Standards** 

· Potyester Powder Coating Aluminium

Polvester Powder Coating Aluminium Minimum Coating Thickness 40 micron 60 micron Non marine environment

BS EN ISO 12206-1:2004

Aggressive environments White coatings Fabrications should be coated to the correct standard and should be

free from dents, scratches. runs and over spray and should have a

## A1S Manufacturing Tolerances Support Components

Brackets & Fittings

Stendard production tolerances as set out in published codes. All brackets and fittings to be manufactured in accordance

with A15 issued drawings and specification.
Discrepancies to be notified to A15 before manufacture.

Hotes positional accuracy
All countersinks to provide flush fitting of lixing +/- 0.5anm

All brackets and filtings to be free from burrs, sharp edges, lase

A1S Manufacturing Tolerances Sections

Angles Guides Box Housing Curtain and Bottom rait

manufacture.

All slock material to conform to all relevant European and British Standard Production Toterances as set out in published codes. All rotter shutter and fire curtain assemblies are manu(actured in accordance with A1S issued drawings and specification. A15 to check all production tolerances are met

Alt discrepancies are to be notified to A1S prior to



















